

# Identification & modeling of complex nonlinear systems from uncertain/limited information

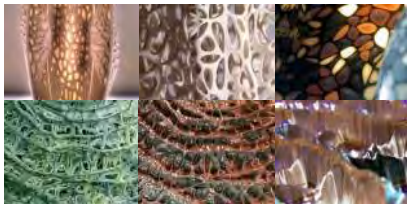
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**Institute of Structural Engineering, ETH Zürich**



**3rd Int. Workshop on Validation of Computational Mechanics Models**  
**June 12, 2014**

# Background & Motivation

## Increased Complexity on the Material & Structural System level



### Functionally Graded Materials

(source: MIT Media Lab)



### Pedestrian Vibrations Millennium Bridge

# Background & Motivation

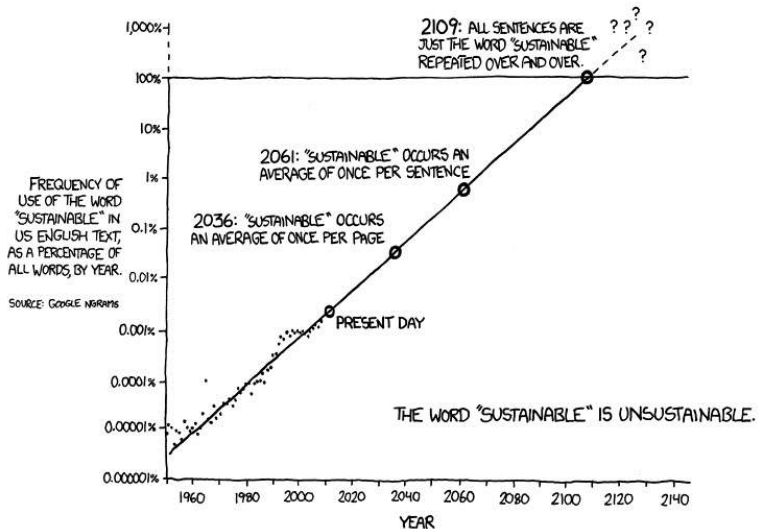
## Deteriorating Infrastructure



### Ageing

Infrastructure demographic

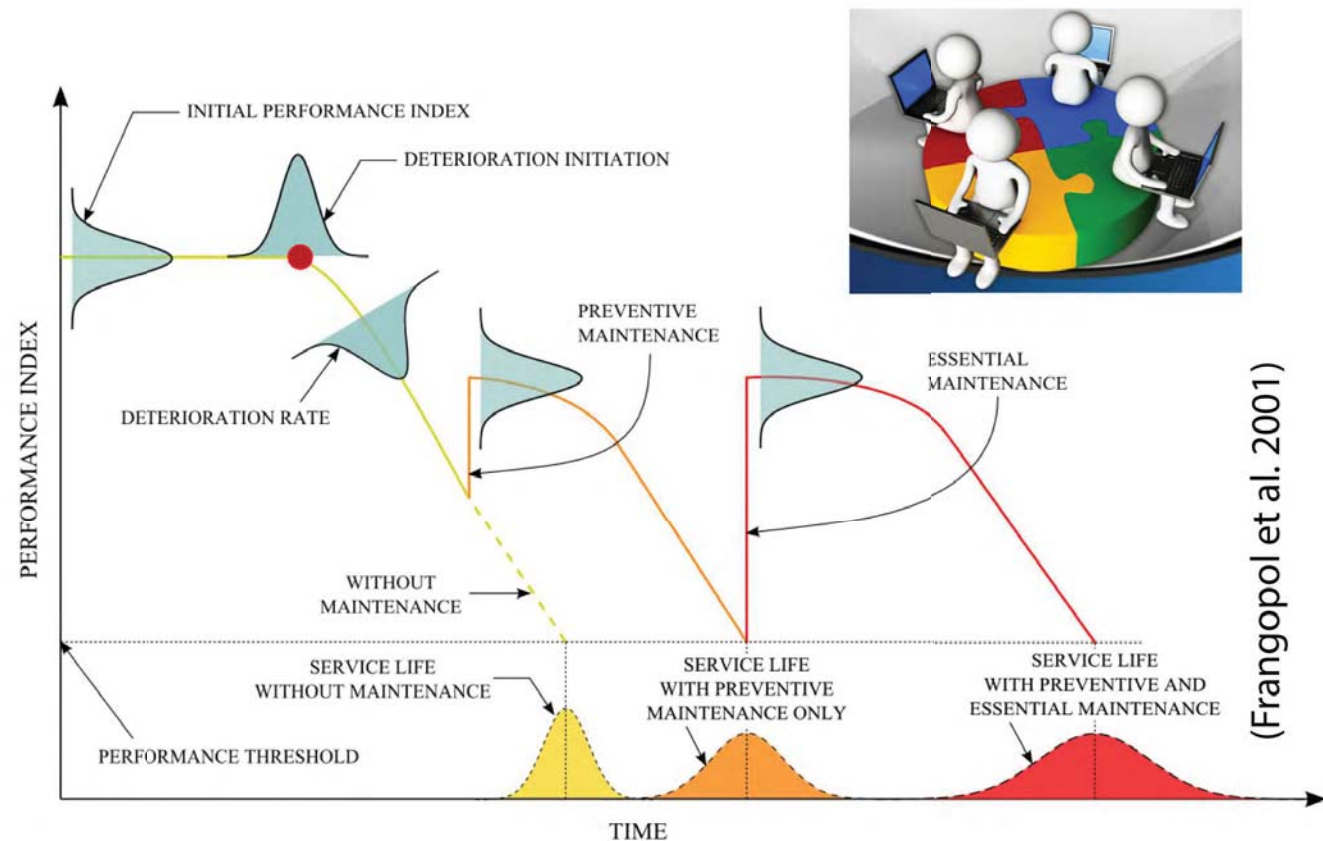
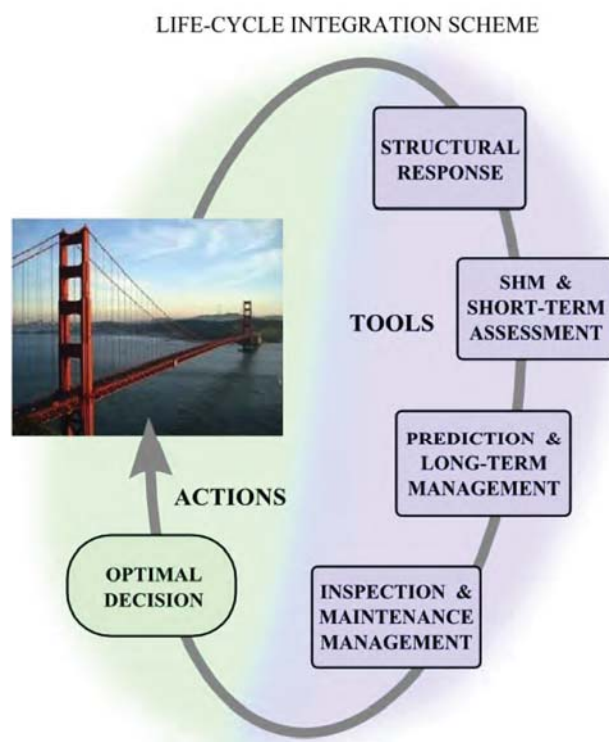
## Quest for Sustainability



(source: xkcd blog)

# Engineering Efficient Infrastructure





## Repairing a Defective System



Exploiting Sensory feedback and appropriately Calibrated Computational models for Infrastructure Efficiency

# The importance of Modeling

## Main Challenges

-  **Incorporating Uncertainties**
-  **Model Validation via Sensory Feedback**
-  **Deliver accurate Reduced Order Models - Metamodels**
-  **Increase Computational Efficiency at a lower Computational toll – the Multiscale formulation**



# Structural Health Monitoring

Sensory feedback via **SHM systems**.

**Force**



**Displacement**



**Strain and tilt**



**Acceleration**



**Meteo**



**Data from GPS**

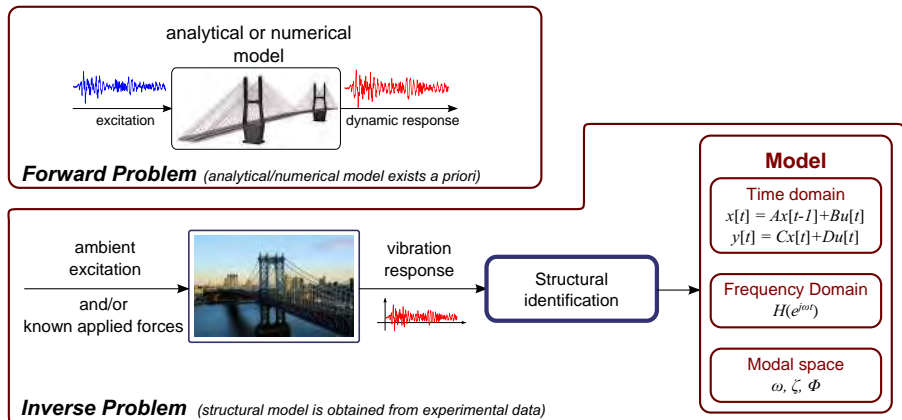


# System Identification in SHM

Link between acquired data & modeling of structural behavior?

## System Identification

Developing or improving the mathematical representation of a physical system using experimental data.





## Reality Check

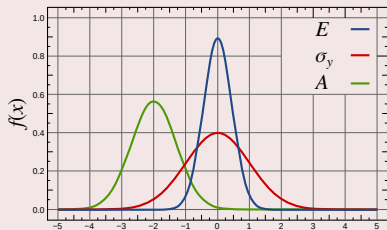
### Challenge #1: Fusion of heterogeneous data, sensor noise



## Reality Check

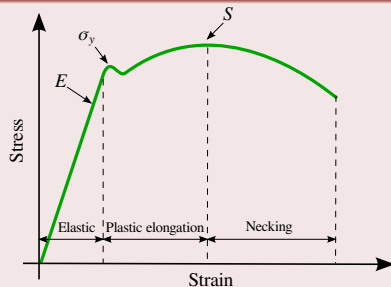
## Challenge #2: Lack of a-priori knowledge of the system itself

Structural system is characterized by parameter uncertainty



The behavior of the modeled structure has to be examined for a **range of structural characteristics**.

Nonlinearities are taken into account



The impact of **different types of excitation** (of different magnitude and/or spectral content) should also be examined.

# Optimal Bayesian Solution

**The Task:** How to estimate  $\mathbf{x}$  given **partial, noisy observations** of the response  $\mathbf{y}$ ?

## Predict

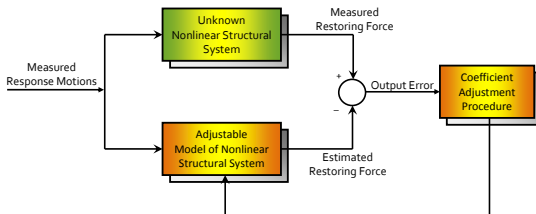
Assuming the prior  $p(x_0)$  is known and that the required pdf  $p(x_{k-1}|y_{1:k-1})$  at time  $k-1$  is available (**Chapman-Kolmogorov equation**):

$$p(x_k|y_{1:k-1}) = \int p(x_k|x_{k-1})p(x_{k-1}|y_{1:k-1})dx_{k-1}$$

## Update

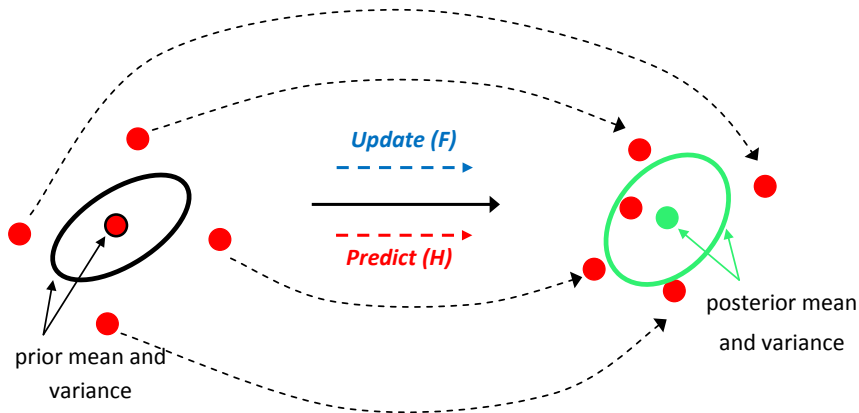
Consequently, the prior (or prediction) is updated using the measurement  $y_k$  at time  $k$  (**Bayes Theorem**):

$$p(x_k|y_{1:k}) = p(x_k|y_k, y_{1:k-1}) = \frac{p(y_k|x_k)p(x_k|y_{1:k-1})}{p(y_k|y_{1:k-1})}$$



# Bayesian Approximation

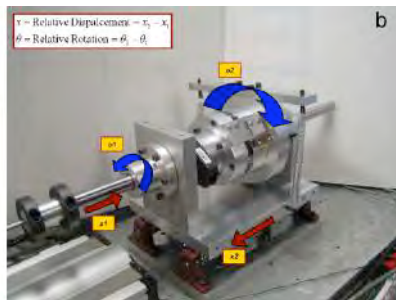
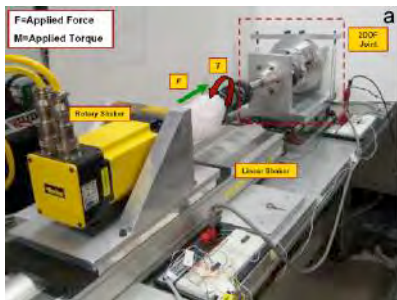
## Particle-based Bayesian approaches - Working principle



Chatzi, E. N. and Smyth, A. W. (2012), "Particle filter scheme with mutation for the estimation of time-invariant parameters in structural health monitoring applications", *Journal of Structural Control and Health Monitoring*.

# Application #1: Treating model Uncertainty in real-time

## On Line Parametric Identification of a Non-Linear Hysteretic System with Model Uncertainty

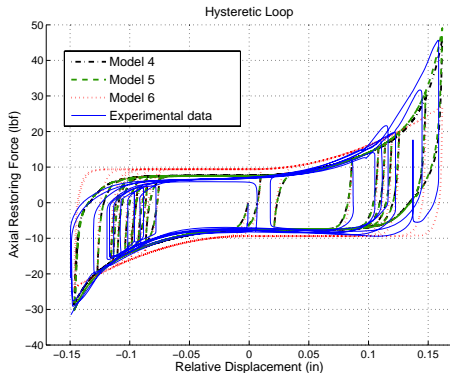
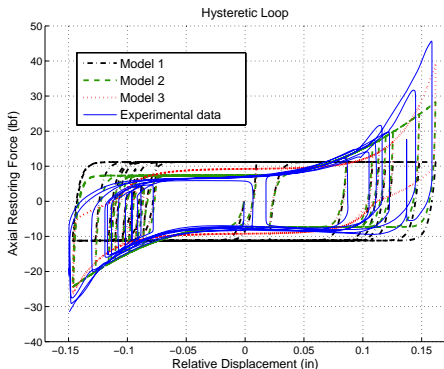


Experimental Setup at USC

E.N. Chatzi, A.W. Smyth and S.F. Masri, "Experimental application of on-line parametric identification for nonlinear hysteretic systems with model uncertainty", *Journal of Structural Safety, Structural Safety*, Vol. 32, No. 5. (24 September 2010), pp. 326-337.

# Nonlinear Hysteretic Joint

## Non typical hysteretic behavior

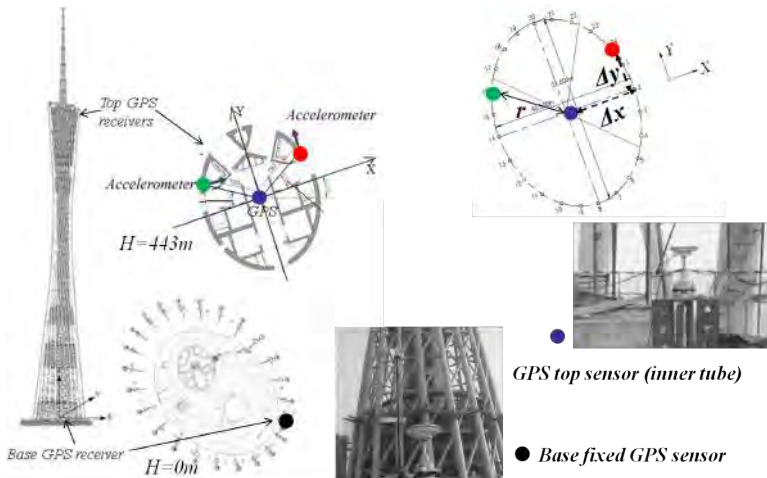


A physical model of the system is formulated **on-the-fly**:

$$\dot{z} = \mathcal{B}_1 \oplus \{c_1 |x|^{m_1} \dot{x}(\text{sgn}(x\dot{x}) + 1)/2 + c_2 \sinh(a_2 x \text{sgn}(\dot{x}))\dot{x}\}, l = 5$$

# Application #2: Actual Large Scale Structure

## Case Study - Tall Tower Structure



E. Chatzi, C. Fuggini, "Structural identification of a super-tall tower by GPS and accelerometer data fusion using a multi-rate Kalman filter", 3th International Symposium on Life-Cycle Civil Engineering (IALCCE 2012), October 3-6, 2012.

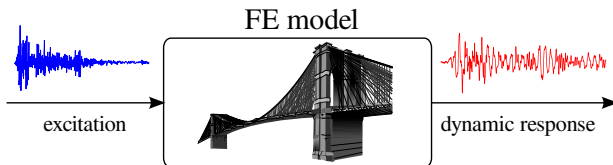
## Quantifying Uncertainty

**Challenge:** How to Quantify Uncertainty through efficient computation?

### Motivation

The simulation of **dynamic response** through FE models requires **excessive computational resources** particularly for complex, large structures. The problem is even more pronounced when:

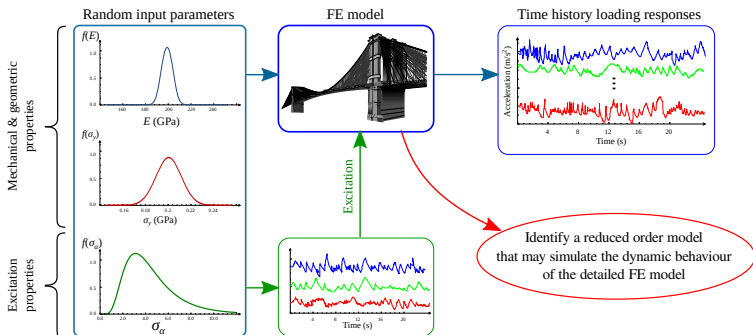
- the structural system is characterized by **parameter uncertainty**
- **detailed geometrical** descriptions are adopted
- **nonlinearities** are taken into account



This is especially important for the case of **inverse problem formulations** or **Reliability Analyses** where a large number of forward runs is necessary.



## One Step Further - The Metamodeling approach



## Problem definition

Consider a structural system represented by a numerical model  $\mathcal{M}$  characterized by uncertain input parameters  $\xi = [\xi_1, \xi_2, \dots, \xi_M]^T$  with known joint pdf  $f(\xi)$ . The **dynamic response** of  $\mathcal{M}$  to a given input excitation  $x[t, \xi]$  will also be a **random variable**:

$$y[t, \xi] = \mathcal{M}(x[t, \xi], \xi), \quad t = 1, 2, \dots, T$$

A **metamodel**  $\tilde{\mathcal{M}}$  must be able to predict and/or simulate the detailed numerical model results in a computationally inexpensive way and with sufficient accuracy.

## PC-ARX Models

## Polynomial Chaos AutoRegressive with eXogenous input (PC-ARX) models

$$\underbrace{y[t] + \sum_{i=1}^{n_a} a_i(\xi) \cdot y[t-i]}_{\text{AR part}} = \underbrace{\sum_{i=0}^{n_b} b_i(\xi) \cdot x[t-i]}_{\text{X part}} + e[t], \quad e[t] \sim \text{NID}(0, \sigma_e^2)$$

AR/X model parameters are modeled as random variables projected on a **polynomial chaos basis**, in order to enable uncertainty propagation.

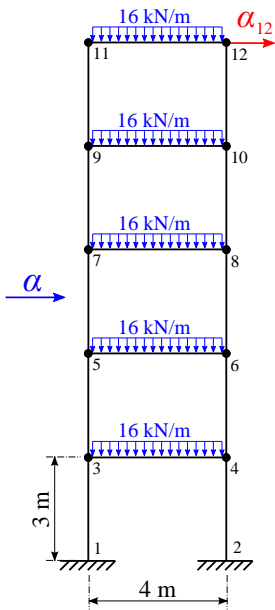
$$a_i(\xi) = \sum_{j=1}^p a_{i,j} \cdot \phi_{d(j)}(\xi),$$

$$b_i(\xi) = \sum_{j=1}^p b_{i,j} \cdot \phi_{d(j)}(\xi)$$

$a_{i,j}, b_{i,j}$ : unknown deterministic coefficients of projection

$\phi_{d(j)}$ : basis functions orthonormal w.r.t. the joint probability density function of  $\xi$

## Implementation on a 5 storey Nonlinear Frame



## Simple Implementation Example

The described framework is implemented for the simulation of the response of a **five-storey shear frame**, subjected to a (known) dynamic input in the form of **earthquake excitation**.

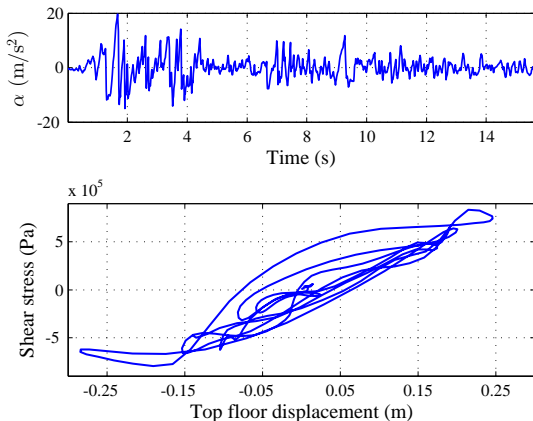
The frame is described by a **nonlinear material law**, allowing for the sections to move into the post-yield region which causes nonlinear behavior to occur.

We consider the following **input parameters**:

Input parameter	Vertical elements	Horizontal elements
Density ( $\text{kg/m}^3$ )	7850	7850
Poisson ratio	0.29	0.29
Young moduli (GPa)	$\mathcal{U}(190, 210)$	$\mathcal{U}(190, 210)$
Yield stress (MPa)	$\mathcal{U}(200, 500)$	$\mathcal{U}(200, 500)$
Cross section area ( $\text{m}^2$ )	$\mathcal{U}(0.04, 0.09)$	0.0625

## Implementation on a 5 storey Nonlinear Frame

One of the recorded acceleration instances for the El Centro earthquake\* has been utilized as ground excitation:



causing the observed **shear stress vs top floor displacement** response.

The curve shown here corresponds to the first simulation experiment (with  $\xi_1$ ) and  $t = 1, 2, \dots, 250$ .

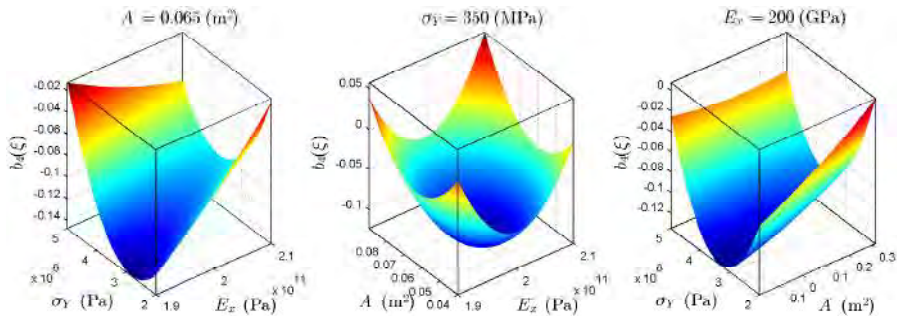
\* downloadable at: [http://peer.berkeley.edu/peer\\_ground\\_motion\\_database](http://peer.berkeley.edu/peer_ground_motion_database)

## Implementation on a 5 storey Nonlinear Frame

20 simulations are conducted using a detailed structural model. The ANSYS finite element software has been used for the reference simulations.

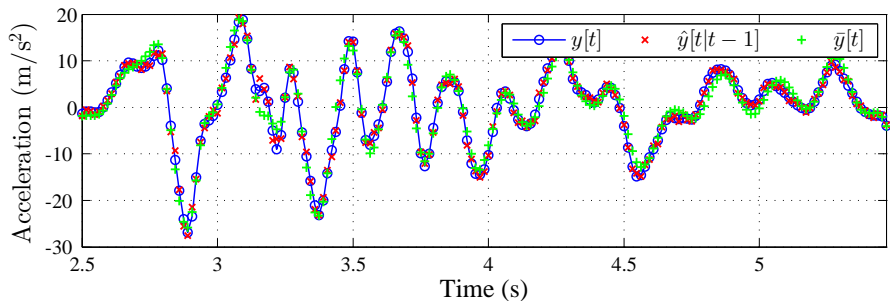
## The derived functional representation

Polynomial expansion of  $b_4(\xi)$  model parameter onto the input space



## Implementation on a 5 storey Nonlinear Frame

In order to **validate** the workings of the metamodeling framework the performance of the identified PC-ARX(10,10) metamodel is tested for the **prediction** and **simulation** of the dynamic response of the FE model subjected this time to the **Pacoima Dam** earthquake.



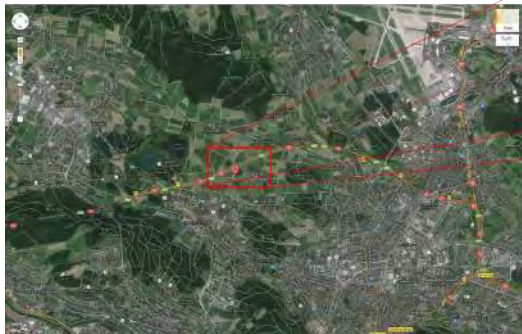
0.7836 % prediction error

3.7585 % simulation error

5000 times reduced simulation time

# Running Field Project

## Structural Identification for Condition Assessment of Swiss Bridges, Research Grant funded by the Federal Roads Office



# Structural Assessment of Complex Structures



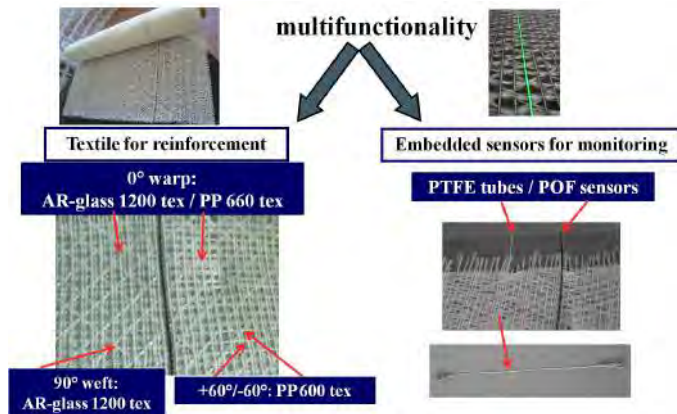
Beyond Reduced Order Models it is often desirable to maintain **model refinement** at a **reduced computational toll**

## Multi - Phase Structures

- Masonry structures constitute a large portion of the existing building stock
- Novel structures, largely based on composite & polymer materials are continuously emerging



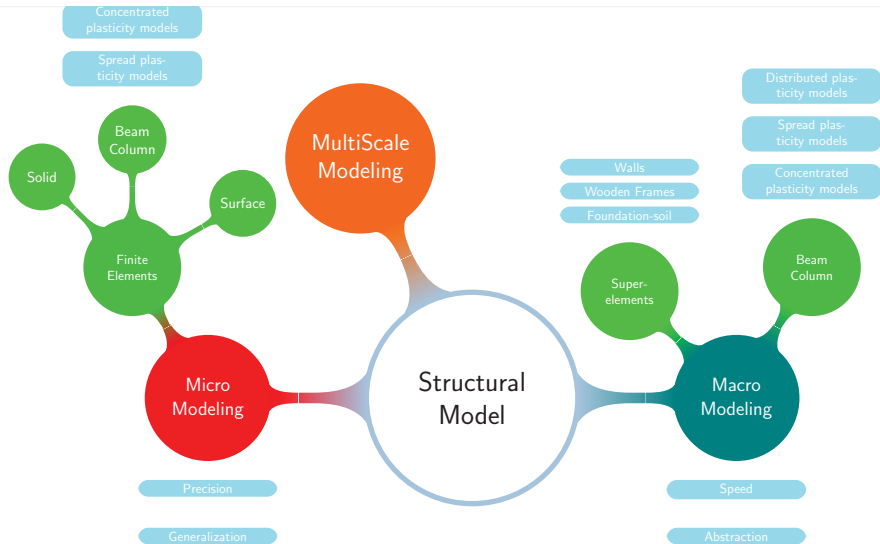
## Implementing novel technologies in masonry retrofitting - SNSF project



## Modeling Challenges

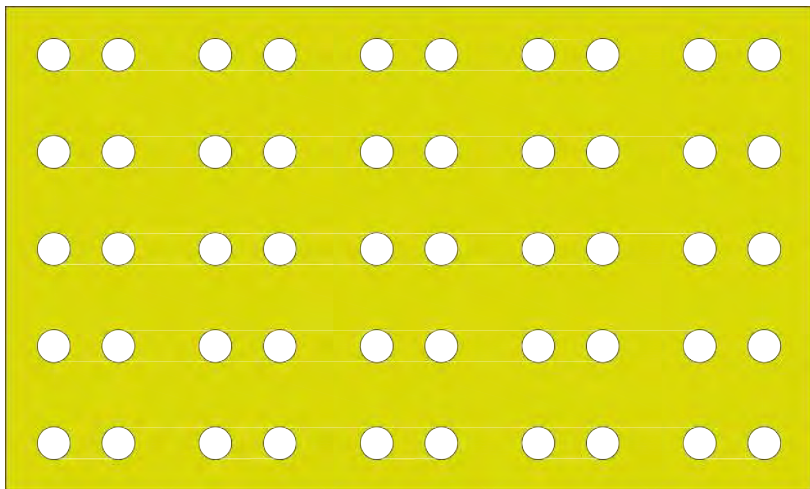
- Complex behaviour of the constituents - Crack propagation
- Material parameters hard to identify
- Standard FEM modeling procedures are too expensive

# Structural Assessment of Complex Structures



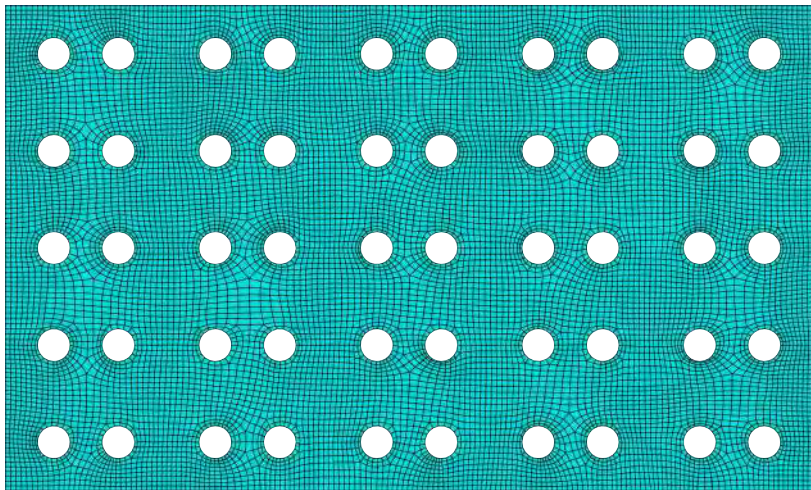
## The Multiscale Finite Element scheme

Given a heterogeneous deformable body (flaws, inclusions, material layers)



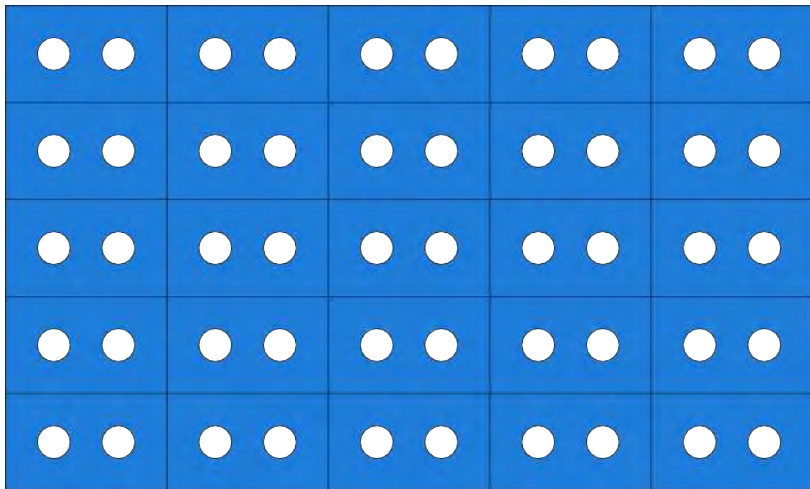
## The Multiscale Finite Element scheme

And an accompanying fine mesh (**standard FE approach**)

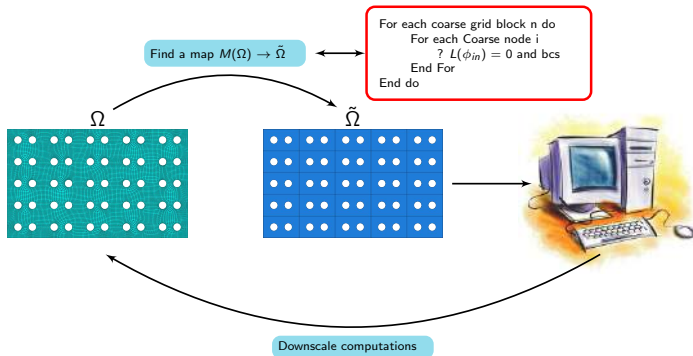


# The Multiscale Finite Element scheme

Solve this mesh instead (**multiscale approach**)



## MultiScale FEM



- Re-evaluation of the mapping is required in a nonlinear analysis
- Use the Hysteretic FE in the micro-scale

## Proposed Approach:

Couple the Multiscale FE approach with a Hysteretic FE Formulation

## The hysteretic formulation of Finite Elements

- Considering the additive decomposition of the strain vector

$$\{\dot{\epsilon}\} = \{\dot{\epsilon}^e\} + \{\dot{\epsilon}^{pl}\}$$

- An evolution equation for the plastic part of total strain is derived

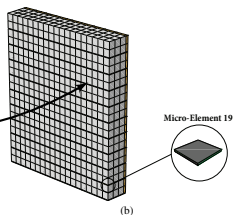
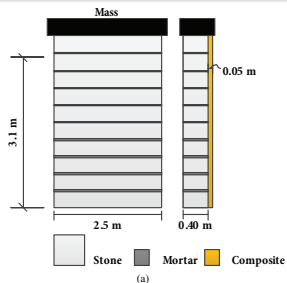
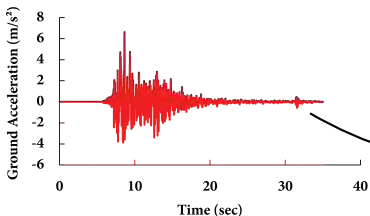
$$\{\dot{\epsilon}^{pl}\} = \left| \frac{\check{\Phi}}{\check{\Phi}_0} \right|^N \left( \beta + \gamma \text{sgn} \left( \{\dot{\epsilon}\}^T \{\sigma\} \right) \right) [R] \{\dot{\epsilon}\}$$

Triantafyllou, S.P., Koumouisis, V.K. (2012). "A Bouc-Wen Type Hysteretic Plane Stress Element", *Journal of Engineering Mechanics*, 138 (3), pp. 235-246

S. Triantafyllou and E. Chatzi (2013), "A novel Hysteretic Multiscale Finite Element Method for Nonlinear Dynamic Analysis of Heterogeneous Structures", 11th International Conference on Structural Safety & Reliability

## Textile Reinforced Masonry Wall

	Mortar	Stone
Young's modulus [MPa]	3494	20200
Poisson's ratio	0.11	0.2
Plasticity	Mohr-Coulomb	von-Mises
Friction angle [deg]	21.8	-
Cohesion [MPa]	0.1	-
Yield Stress [MPa]	-	69.2

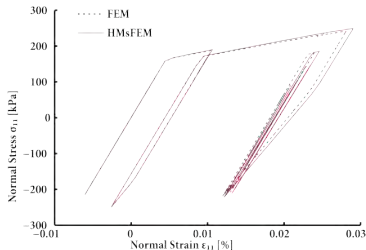
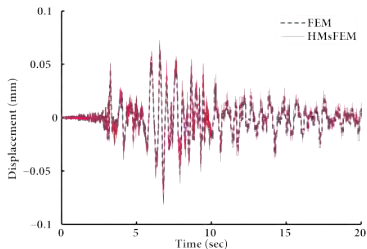


	Composite Material		
Young's modulus [MPa]	$E_{11} = 54000$	$E_{22} = 53200$	$E_{33} = 53200$
	$E_{12} = 53200$	$E_{23} = 54000$	$E_{23} = 54000$
Poisson's ratio	$\nu_{12} = 0.14$	$\nu_{23} = 0.2$	$\nu_{13} = 0.2$



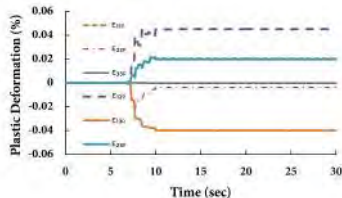
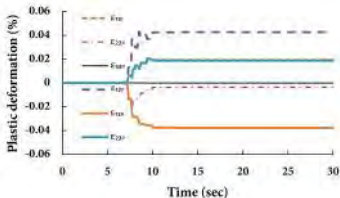
# Textile Reinforced Masonry Wall

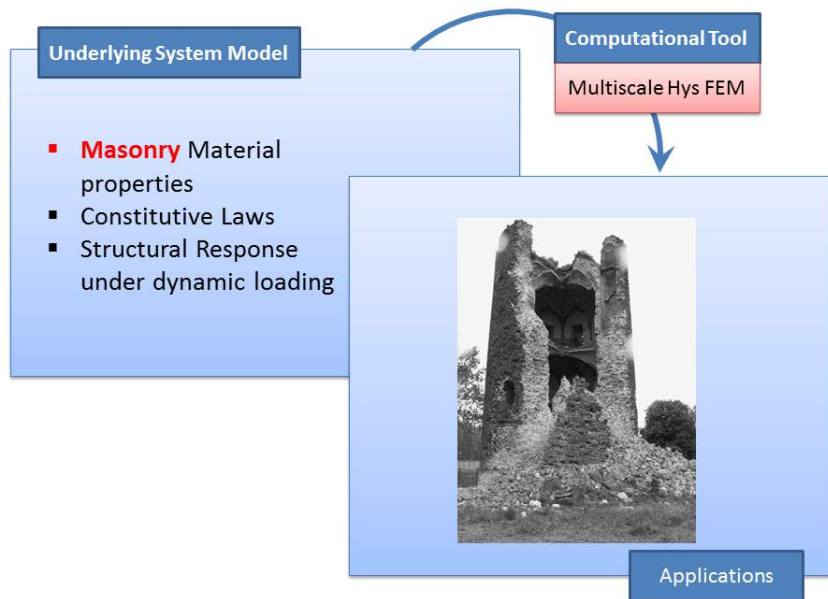
- Results derived from the HMsFEM formulation are compared to classical FEM



75% Reduction in Computational Time

- The plastic strain components are readily derived as part of the solution





A **computational analysis** framework for **dynamically evolving** systems

Underlying System Model

- **Novel** Material properties
- Constitutive Laws
- Structural Response

Computational Tool

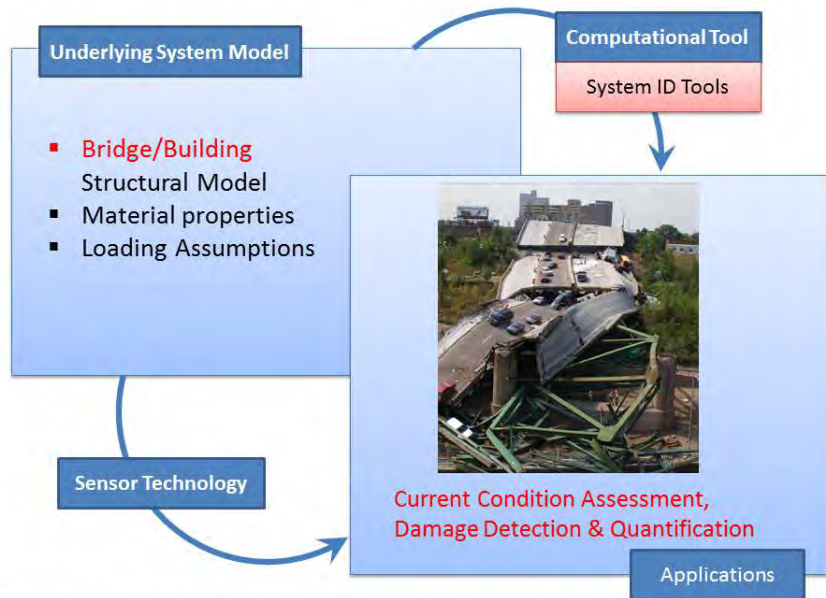
Multiscale Hys FEM



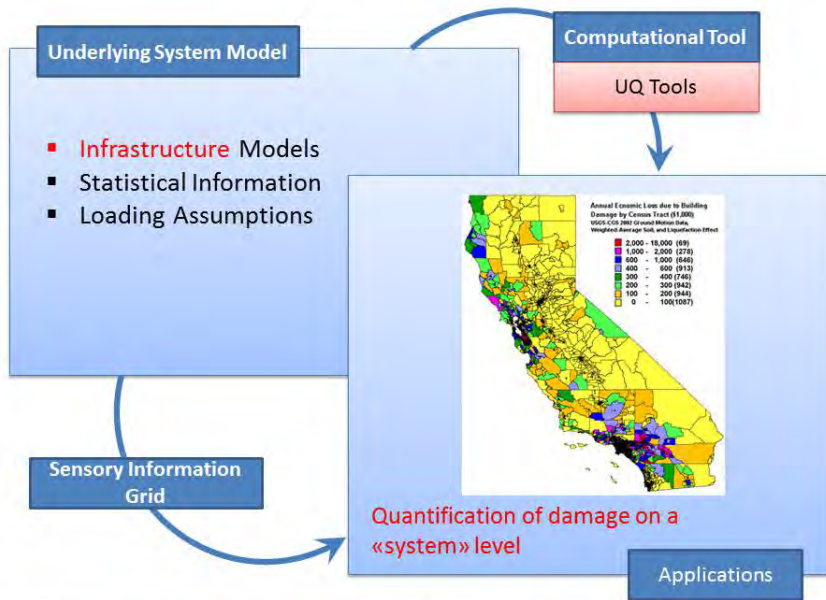
Ultra High Performance Fiber  
Reinforced Concrete

Applications

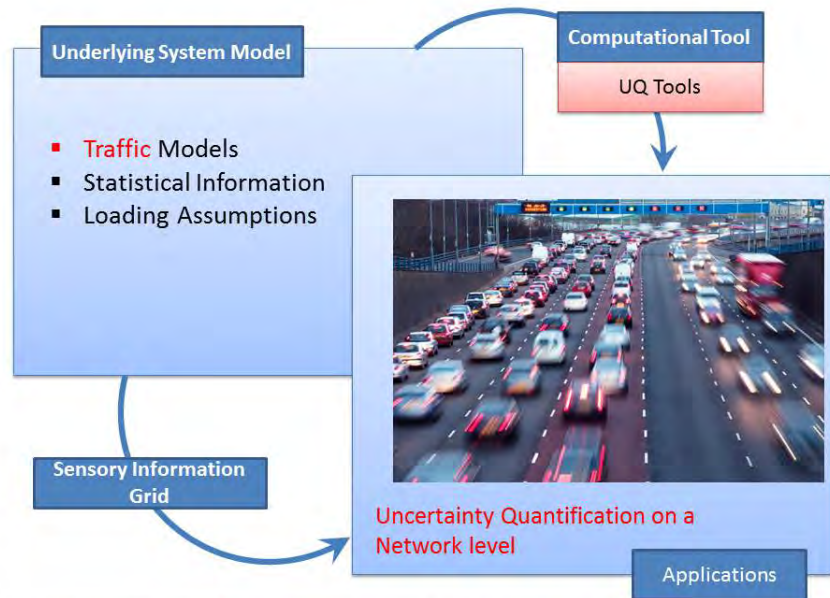
A **computational analysis** framework for **dynamically evolving** systems



A **computational analysis** framework for **dynamically evolving** systems



**A computational analysis framework for dynamically evolving systems**



A **computational analysis** framework for **dynamically evolving** systems

# The Broad Picture

## A gathering of Competences

